THE EFFECT OF PLASTIC DEFORMATION ON THE STRAIN ENERGY RELEASE RATE IN A CENTRALLY NOTCHED PLATE SUBJECTED TO UNIAXIAL TENSION

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FOREWORD

This report was written by R. G. Forman, Aerospace Engineer, Theoretical Mechanics Branch, Structures Division, Air Force Flight Dynamics Laboratory. The work was initiated under Project No. 1467, "Structural Analysis Methods," Task No. 146704, "Structural Fatigue Analysis."

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This technical report has been reviewed and is approved.

FRANCIS J. JANIK, JR.

Chief, Theoretical Mechanics Branch

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ABSTRACT

By using the Dugdale model for a crack in a plate, an improved formula was derived for the strain energy release rate, G. The formula has the same form as the solution for a linear elastic plate, except a correction factor is used which corrects for both the effect of yielding and the finite width of the plate. Curves are presented giving the values of the correction factor, and they indicate that the nominal stress to yield stress ratio has a pronounced effect on the strain energy release rate.

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LIST OF SYMBOLS

A	Crack surface area
$A_{\mathbf{g}}$	Plate gross cross sectional area
A _n	Plate net cross sectional area
В	Plate width
E	Young's modulus
G	Strain energy release rate
$G_{\mathbf{c}}$	Strain energy release rate at onset of fast fracture
K	Plate axial rigidity. Stress intensity factor
L	Plate length
P	End load on plate
P'	Total load across net section
T	Applied tension stress normal to crack
U	Elastic energy in plate
Y	Tensile yield stress
a	Half crack length
c	a + ρ
е	Natural base of logarithms
e _o	Given extension of plate.
k	2a/B , Ratio of crack length to plate width
t	Plate thickness
v	Displacement of crack boundary
v _a	Displacement at crack tip
v _o	Displacement at center of crack
x, y	Cartesian coordinates
σ	Stress in y direction

LIST OF SYMBOLS (Cont'd.)

ρ	Yield zone length
β	πT/2Y
α	Arccosh $\frac{x}{c}$. Strain energy release rate correction factor
γ	Strain energy release rate correction factor

SECTION I

INTRODUCTION

In fracture mechanics theory, G is defined as the plane stress value for the strain energy release rate with crack extension per unit of crack surface area. At onset of unstable crack propagation, the critical strain energy release rate, $G_{\rm c}$, is approximately equal to the plastic energy dissipation rate in the yielded region at the crack tip. The size of the yielded region is assumed to be so small that the elastic stress distribution is unaltered by the presence of the yield zone. Then the strain energy release rate can be calculated by linear elastic analysis methods. Futhermore, $G_{\rm c}$ is assumed to be a material parameter governing the fracture toughness of the material or the resistance to crack extension.

Unfortunately, in actual practice, G_C has been found to vary with specimen dimensions and with nominal (gross) stress levels. Lorenz (Reference 1) found a definite effect of nominal test stress levels upon measured fracture toughness of centrally cracked panels. Kraft, Sullivan, and Boyle (Reference 2) introduced the concept of crack extension resistance, R, and discussed the variation in R with specimen width and crack length.

To progress with the use of G in fracture mechanics problems it is necessary to derive a more accurate solution for the strain energy release rate which takes into account the plastic deformation at the crack tip. The current practice to correct for the effect of yielding is to arbitrarily assume an increased crack length which extends into the yield zone. The yield zone size is determined by applying a yield criteria to the linear elastic stress field equations. The object of this report is to derive a more reasonable solution for the strain energy release rate by using a model which does not assume that the elastic stress distribution is unaltered by the presence of the yield zone. This is accomplished by using the Dugdale (Reference 3) model for which the yield zone size is formulated from equilibrium considerations. With the Dugdale model, G is obtained by using a method derived by Irwin and Kies (Reference 4). A tension load, P, is assumed to be applied to the plate, and a strain energy function is derived from Greenspan's (Reference 5) formulation for the axial rigidity of the plate. The strain energy function is differentiated with respect to the crack surface area to obtain the strain energy release rate.

SECTION II

PROBLEM SOLUTION

1. ADOPTION OF DUGDALE'S MODEL

Dugdale's model for a crack in tension under plane stress is shown in Figure 1(a). The model, derived from the mathematical developments of Muskhelishveli (Reference 6), is based on the following assumptions:

- 1. Yielding occurs in a narrow wedge shaped zone.
- 2. The material in the zone is under a uniform tensile yield stress, Y.
- 3. A Tresca yield criterion is obeyed.
- 4. The material outside the zone is elastic and bounded internally by a flattened ellipse of length $2(a + \rho)$ where a is the half-length of the crack, and ρ is the length of the plastic extension.
- 5. The length ρ is such that there is no stress singularity at the ends of the flattened ellipse.

By determining the plastic zone size in equilibrium with the applied stress, Dugdale obtained the following solution for the plastic extension:

$$\rho = a (\sec \beta - 1) \tag{1}$$

where $\beta = \frac{\pi}{2} \frac{T}{Y}$ and T is the applied tension stress normal to the crack.

Agreement between the observed and calculated values of ρ has been exceedingly good. Dugdale's experimental results showed good agreement with theory. Goodier and Field (Reference 7) referred to numerous experimenters who have observed the Dugdale type yield zone. Hahn (Reference 8) has obtained good agreement with theoretical ρ in tests on thin plates of silicon steel in which the yield zones appeared as those shown in Figure 1(b). Hahn (Reference 9) also determined that the yield zone configuration begins to approach a narrow, tapered Dugdale type when

$$\rho \sim 4t$$
 (2)

where t is the plate thickness. Finally, this writer has observed close agreement with Equation 1 in tests of 0.02 inch thick sheets of AM350 and AM355 steel in which the yield zones were also similar in shape to the one shown in Figure 1(b).

Hahn (Reference 9) programmed Dugdale's stress-field solution for a computer and determined that some terms in the solution were negligibly small. Ignoring these terms, the stress gradient is described by the equation

where σ is the stress in the y direction,

$$\alpha = \operatorname{arc \; cosh} \frac{x}{c}$$
 and $c = a + \rho$.

The displacement of the crack boundaries has been derived by Goodier and Field (Reference 7) for the Dugdale model and is given by the equation

$$v = \frac{cY}{\pi E} \left[\cos \theta \, \mathcal{L}_{\mathcal{R}} \left(\frac{\sin^2 (\beta - \theta)}{\sin^2 (\beta + \theta)} \right) + \cos \beta \, \mathcal{L}_{\mathcal{R}} \left(\frac{(\sin \beta + \sin \theta)^2}{(\sin \beta - \sin \theta)^2} \right) \right] \tag{4}$$

where

Poisson's ratio is taken as 1/3.

v is the displacement in the y direction.

E is Young's modulus.

$$\theta = \operatorname{arc} \cos \frac{x}{c}$$

At the center of the crack (x = 0), Equation 4 reduces to the form

$$v_{o} = \frac{T}{E} \frac{a}{2\beta} \mathcal{L}_{n} \left(\frac{\sin \beta + 1}{\sin \beta - 1} \right)^{2}$$
 (5)

At the tip of the crack (x = a), Equation 4 reduces to

$$v_{a} = \frac{T}{E} \frac{2a}{\beta} \ln \sec \beta \tag{6}$$

Hahn (Reference 9) experimentally measured v_a , and found good agreement with Equation 6.

Equations 3, 4, 5, and 6 make up the solutions for the stress gradient and displacements which are required in the next section to derive the axial rigidity expression.

2. CALCULATION OF THE AXIAL RIGIDITY

Using the reciprocal theorem in elasticity on a perforated and an unperforated plate, Greenspan derived the following expression for the axial rigidity of a perforated plate subjected to uniaxial tension:

$$K = \frac{1}{1 + \frac{2Et}{PL} \int u_p \, dx}$$
 (7)

where

the coordinates x and y are in the width and length directions, respectively, as shown in Figure 1(a).

K is the axial rigidity defined as the ratio of the overall extension of the unperforated plate to that of the perforated plate.

P is the uniaxial tensile force applied to the plate.

L is the length of the plate.

u is the y-direction displacement along the perforation which is integrated along the hole boundary.

As shown in Equation 7, and load, P, and the integral of the displacements along the perforation must be known to determine the axial rigidity, K. Using Equation 3 to calculate P and Equations 4, 5 and 6 to calculate the integral of the displacements, the following expression for the axial rigidity was obtained:*

$$K = \frac{1}{1 + \frac{\pi B}{2!} - \frac{C(v)}{n^2 C(n)}}$$
 (8)

where

$$C(v) = \frac{Y}{T} \left[\ln \sec \beta + \left(\frac{\arcsin p}{4p} \right) \ln \left(\frac{\sin \beta + 1}{\sin \beta - 1} \right)^{2} \right]$$

$$p = \left[1 - \left(\frac{4 \ln m}{\ln \left(\frac{\sin \beta + 1}{\sin \beta - 1} \right)^{2}} \right)^{2} \right]^{1/2}$$

$$N = n + \sqrt{n^2 - m^2} \quad , \quad m = \sec \beta$$

 $n = \frac{B}{2a}$ is the ratio of plate width to total crack length.

(a) For
$$0 < \frac{T}{Y} < 0.5$$
,
$$C(n) = \frac{(m-1)}{n} \frac{Y}{T} + \frac{(n-m)}{n} + \frac{m \tan 2\beta}{2n\beta} \left[1 - \frac{m}{N} - \frac{\sin^2 \beta}{\sqrt{\cos 2\beta}} \ln \frac{(N-m\sqrt{\cos 2\beta})(1 + \sqrt{\cos 2\beta})}{(N+m\sqrt{\cos 2\beta})(1 - \sqrt{\cos 2\beta})} \right]$$
(b) For $0.5 < \frac{T}{Y} < 1$,

$$C(n) = \frac{(m-1)}{n} \frac{Y}{T} + \frac{(n-m)}{n} + \frac{m \tan 2\beta}{2n\beta} \left[1 - \frac{m}{N} - \frac{2 \sin^2 \beta}{\sqrt{-\cos 2\beta}} \arctan \left(\frac{(N-m)\sqrt{-\cos 2\beta}}{N - m \cos 2\beta} \right) \right]$$

^{*} Some details of this computation are shown in Appendixes I and II.

3. STRAIN ENERGY RELEASE RATE

If a given tension load is applied to a centrally cracked plate, the stored elastic energy is inversely proportional to the axial rigidity, K. Following a development similar to Irwin's, it is necessary to assume an initial load P_{o} which would prevail for a given extension e_{o} of the plate if the central crack did not exist. Thus, for a plate of thickness t,

$$e_0 = \frac{P_0 L}{EBt} = \frac{PL}{KEBt}$$
 (9)

where P is any tensile load supported by a plate containing a transverse central crack. For plane stress, the elastic energy is

$$U = \frac{1}{2} \frac{P^{2}L}{KEBt} = \frac{1}{2} e_{0}^{2} \frac{EBt}{L} K$$
 (10)

Letting k = 1/n = 2a/B, then for fixed ends,

$$\frac{dU}{dk} = \frac{1}{2} e_0^2 \frac{EBt}{L} \frac{dK}{dk}$$
 (11)

Or, since the crack surface area, A, is equal to Btk,

$$\frac{dU}{dA} = \frac{1}{Bt} \frac{dU}{dk} = \frac{1}{2} \frac{T^2L}{E} \frac{dK}{dk}$$
 (12)

where $T = P_O / (Bt)$.

Finally, using the expression for K in Equation 8, the strain energy release rate for the Dugdale model is

$$\frac{dU}{dA} = -\frac{\pi T^2 \sigma}{E} \left[\frac{C(v)}{2} \frac{\left[1 + C(n) + \frac{\tan \beta}{\beta(nN-1)}\right]}{\left[\frac{B}{L}C(v)k^2 - C(n)\right]^2} \right]$$
(13)

If L>> B which is a condition satisfied in many instances, Equation 13 simplifies to

$$\frac{dU}{dA} = G = -\frac{\pi T^2 a}{F} \gamma^2 \tag{14}$$

where

$$\gamma^2 = \frac{C(v)}{2C(n)^2} \left[1 + C(n) + \frac{\tan \beta}{\beta(nN-1)} \right]$$

Equation 14 has the same form as the solution for a linear elastic plate, except that γ is a correction factor which corrects for both the finite width of the plate and the effect of the plastic deformation at the crack tip. To aid in using the correction factor, values of γ are listed in Table 1 for appropriate ranges of T/Y and k.

To show the significance of Equation 14, the equation can be compared with the extended crack solution reported in Reference 10. The extended crack solution can be written as follows:

$$G = \frac{K^2}{E}$$
 (15)

where in this equation, K is the stress intensity factor expressed as

$$K^2 = T^2 B \tan \left(\frac{\pi a}{B} + \frac{K^2}{2BY^2} \right)$$

For a first approximation, if it is assumed that $K^2 = \pi T^2$ a α^2 , Equation 15 becomes

$$G = \frac{\pi T^2 a}{E} a^2$$
 (16)

where

$$\alpha^2 = \frac{2}{\pi k} \tan \left\{ \frac{\pi k}{2} \left[1 + \frac{1}{2} \left(\frac{T \alpha}{Y} \right)^2 \right] \right\}$$

For comparison, the values of α and γ are plotted in Figure 2 for several ratios of T/Y. This figure shows that at higher ratios of T/Y, the extended crack solution gives significantly lower values for G than the improved solution based on the Dugdale model. Also of particular interest is the difference in α and γ when T/Y is zero. The reason for this is because α is a solution for equally spaced collinear cracks in an infinite plate. If the values of γ for T/Y equal to zero are compared with Dixon's (Reference 11) correction factor for a finite width plate, the agreement is excellent.

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APPENDIX I

INTEGRATING THE CRACK BOUNDARY DISPLACEMENTS

Due to complications in integrating Equation 4 directly, an indirect but accurate method of integration was used. If v is plotted along the axis of the crack from x = -a to x = a, the resulting curve has an elliptical shape. This curve can be generally approximated within three significant figures by the following equation in which f is determined from the condition that $v = v_a$ at x = a.

$$\frac{v^2}{v_0^2} + \frac{x^2}{f^2} = 1 \tag{17}$$

where

v_o is obtained from Equation 5.

v_a is obtained from Equation 6.

$$f = \frac{q}{\left[1 - \left(\frac{4 \ln \sec \beta}{\ln \left(\frac{\sin \beta + 1}{\sin \beta - 1}\right)^2}\right)^2\right]^{1/2}}$$

The area of the shaded segment of the ellipse shown in Figure 3 gives the value of the desired definite integral. The integral is

$$\int_{-a}^{a} v dx = av_{a} + fv_{o} \arcsin \frac{a}{f}$$

$$= \pi a^{2} \frac{T}{F} C(v)$$
(18)

where

$$C(v) = \frac{Y}{T} \left[ln \sec \beta + \left(\frac{\arcsin p}{4p} \right) ln \left(\frac{\sin \beta + 1}{\sin \beta - 1} \right)^{2} \right]$$

$$p = \left[1 - \left(\frac{4 ln \sec \beta}{ln \left(\frac{\sin \beta + 1}{\sin \beta - 1} \right)^{2}} \right)^{1/2} \right]^{1/2}$$

Equation 18 is the solution for an infinite plate with the correction factor C(v) accounting for the affect of the yield deformation. For comparison, values of C(v) are listed below for values of T/Y ranging from 0 to 0.8.

T/Y	C(v)
0	1
0.2	1.028
0.4	1.115
0.6	1.288
0.8	1.661

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As just mentioned, Equation 18 is for a crack in an infinite plate and then should probably not be used for k > 0.5.

APPENDIX II

CALCULATING THE LOAD, P

Using a method similar to Greenspan's, the load, P, can be determined by integrating the axial stresses along the x axis from x = 0 to the plate edge, x = na. The symbols used for the derivation are shown in Figure 4. The total load carried by the plate is

$$P' = 2t \int_{0}^{na} \sigma_{y} dx$$

$$= 2Y ta(m-1) + 2t \int_{ma}^{na} \sigma_{y} dx$$
(19)

where

$$\sigma_y = T + \frac{T}{\beta} \arctan \left(\frac{\sin 2\beta}{e^2 - \cos 2\beta} \right)$$

$$e^{2\alpha} = \frac{1}{c^2} \left(x + \sqrt{x^2 - c^2} \right)$$

A useful simplification in integrating the stresses can be obtained by making the following assumption:

$$\int_{ma_{1}}^{na} \arctan\left(\frac{c^{2} \sin^{2} \beta}{(x + \sqrt{x^{2} - c^{2} \cos 2\beta})^{2} - c^{2} \cos 2\beta}\right) dx$$
 (20)

$$\approx \int_{ma}^{na} \frac{c^2 \sin^2 \beta dx}{(x + \sqrt{x^2 - c^2 \cos 2\beta})^2 - c^2 \cos 2\beta}$$

This simplification was checked numerically, and for the ranges required for T/Y and k, the maximum error that resulted was less than 3 percent.

The integration can be performed by making the following change in variables:

$$w = x + \sqrt{x^2 - c^2}$$
, $dx = \frac{1}{2} \left(1 - \frac{c^2}{w^2}\right) dw$ (21)

Since $\cos 2\beta$ is positive for O < T/Y < 0.5 and negative for 0.5 < T/Y < 1, two solutions are required for the integration. After performing the integration, the total load is as follows:

(a) For
$$0 < \frac{T}{Y} < 0.5$$
,

$$P' = 2at T\left(\frac{Y}{T}\right) (m-l) + 2at T(n-m)$$

$$+\frac{2\operatorname{atm} T \tan 2\beta}{2\beta} \left[1 - \frac{m}{N} - \frac{\sin^2 \beta}{\sqrt{\cos 2\beta}} \int_{\mathbb{N}} \left(\frac{(N - m \sqrt{\cos 2\beta}) (1 + \sqrt{\cos 2\beta})}{(N + m \sqrt{\cos 2\beta}) (1 - \sqrt{\cos 2\beta})} \right) \right]$$
 (22)

(b) For
$$0.5 < \frac{T}{Y} < 1$$
,

$$P' = 2 \operatorname{at} T \left(\frac{Y}{T} \right) (m-1) + 2 \operatorname{at} T (n-m)$$

$$+ \frac{2 \operatorname{at} m T \tan 2\beta}{2\beta} \left[1 - \frac{m}{N} - \frac{2 \sin^2 \beta}{\sqrt{-\cos 2\beta}} \left(\arctan \frac{N}{m\sqrt{-\cos 2\beta}} - \arctan \frac{1}{\sqrt{-\cos 2\beta}} \right) \right] (23)$$

where

$$N = n + \sqrt{n^2 - m^2}$$

But, the end load, P, may be substituted for the total load across the net cross section, P^{\prime} , if

$$n = \frac{Ag}{A_g - A_n} = \frac{Ag}{2at} \tag{24}$$

where

 A_g is the plate gross cross sectional area at y = L/2.

 A_n is the plate net cross sectional area at y = 0.

Making this substitution, the total load, P, is

$$P = TA_g C(n)$$
 (25)

where

(a) For
$$0 < \frac{T}{V} < 0.5$$
,

$$C(n) = \frac{(m-1)}{n} \frac{Y}{T} + \frac{(n-m)}{n} + \frac{m \tan 2\beta}{2n\beta} \left[1 - \frac{m}{N} - \frac{\sin^2 \beta}{\sqrt{\cos 2\beta}} \right] \ln \frac{(N-m\sqrt{\cos 2\beta})(1+\sqrt{\cos 2\beta})}{(N+m\sqrt{\cos 2\beta})(1-\sqrt{\cos 2\beta})}$$

(b) For
$$0.5 < \frac{T}{Y} < 1$$
,

$$C(n) = \frac{(m-1)}{n} \frac{Y}{T} + \frac{(n-m)}{n} + \frac{m \tan 2\beta}{2n\beta} \left[1 - \frac{m}{N} - \frac{2 \sin^2 \beta}{\sqrt{-\cos 2\beta}} \arctan \frac{(N-m)\sqrt{-\cos 2\beta}}{N-m \cos 2\beta} \right]$$

When T/Y = O, the expression for C(n) reduces to the form:

$$C(n) = 1 - \frac{k^2}{1 + \sqrt{1 - k^2}}$$
 (26)

It is interesting to compare Equation 26 with Greenspan's solution for C(n) used in Reference 4 to obtain the strain energy release rate. This solution is

$$C(n) = 1 - \frac{1}{2} k^2 - \frac{1}{2} k^4$$
 (27)

For equal values of k, Equation 27 gives slightly lower values for C(n) than does Equation 26. The reason for this is that Equation 27 was actually derived for a circular hole in a plate and Equation 26 was derived for an elliptical hole.

TABLE I
VALUE OF CORRECTION FACTOR, γ

0.10	0	1.00305	1 00290	0031	33	0036	1.00409	1.00463	1.00526	1.00600	1.00685	1.00780	1.00886	1.01003	1.01131	1.01270	1.01420	1.01582	1.01/56	1.01942	1.02140	1.02350	1.02573	1.02809	1.03059	1.03322	1.03599	16860-1	1.04197	1.04518	1.04856	1.05209	1.0507	1 04371	1.06371	1.07237	1.07699	1.08182	1.08687	O.	1.09764	1.10338	1.10937	1.11563	1.12217	1.12900	1.13614	1.14359
60.0	1.00258	1.00240	6200	1.00249	0027	0030	0034	1.00403	2	-	1.00627	1.00723	1.00829	000	1.01075	1.01215	1.01365	1.01528	70/10-1	1.01888	1.02087	1.02297	1.02521	1.02757	1.03007	.0327	1.03548	1.03840	.0414	0446	1.04805	0515	2000	1660	1.06744	0718	0764	0813	1.08637	9160	1260	028	88	151	1.12165	284	1.13561	_
0.08		1.00183	1.0017	200	0021	I IO	29	10	ユ	6	• 0058	~	.0078	1.00902	.0103	.0117	1.01324	•0148	9910	84	.0204	1.02260	1.02484	1.02721	1.02972	1.03236	1.03514	1.03806	1.04114	1.04436	1.04774	.0512	1.00409	00000	N —	1.07160	076	.0810	_	13	1.09688	.0	36	m		m	10	1.14284
0.07	1.00148	1.00134	1.00129	1.00150	1.00175	1.00211	1.00257	1.00313	1.00380	1.00456	1.00544	1.00642	1.00751	1.00870	1.01001	1.01142	1.01296	1.01460	1.01636	1.01825	1.02025	1.02238	1.02463	1.02702	1.02954	1.03219	1.03499	1.03/92	1.04101	1.04424	1.04764	1.05119	1.05491	1.02880	1 06207	4 10	N 04	90	0861	9160	1.09693	026	087	149	15	283	355	429
90.0	1.00105	6000	1 00004	.0011	0	.0017	.0022	.002	.0035	.0043	0052	.0062	.007	1.00852	• 0098	.011	.0128	1.01449	1.01627	1.01817	1.02020	1.02234	1.02462	1.02702	1.02956	1.03223	1.03504	1.03800	1.04110	1.04436	1.04777	1.05134	0250	1.05900	1 04734	1.07183	1.07649	13	1.08645	.0917	0	030	1:10912	154	2	28	36	1.14353
0.05	1.00068	.0005		1.00087			1.00207		1.00337	•	1.00510		•	•	•		1.01287	•	1.01636	•	•	•		•	1.02980	•	1.03533	•	•	1.04473	•	1.05177		•	•		1.07710		•	•		1.10386	7	1.11627	.1228	.1297	1.13701	1.14454
0.04	1.00040	1.00032	1.00057	1.00070	1.00103	1.00146	1.00199	1.00262	1.00336	1.00420	1.00515	1.00620	1.00736	1.00862	1.01000	1.01149	1.01310	1.01482	1.01665	1.01867	1.02069	1.02290	1.02523	1.02769	1.03029	1.03302	1.03589	1.03891	1.04208	1.04540	1.04888	1.05252	1.05633	1.000.1	1 06887	1.07335	1.07809	1.08304	1.08820	1.09359	1.09922	1.10508	1.11121	1.11760	1.12427	1.13123	1.13850	1.14610
0.03	1.00019	•	12000-1	1.00054	1,00101		00.	00.	00.	00.	1.00535	1.00644	1.00764	1.00895	1.01037	1.01190	1.01354	1.01530	1.01/16	1.01918	1.02130	1.02355	1.02592	1.02843	1.03107	1.03385	1.03677	1.03983	1.04305	1.04642	1.04994	1.05363	1.05/49	1 06572	1 07014	1.07474	1.07954	.08	089	.09	.10	.10		.11	.12	.13	40	1.14829
0.02	1.00005	0000	1.00018	7000	001	1.00163	1.00225	1.00297	1.00379	1.00472	1.00575	.0068	1.00814	1.00950	.0109	1.01255	.0142	1.01605	•01/9	1.02004	1.02221	1.02451	1.02694	1.02951	1.03220	1.03504	1.03802	1.04114	1.04442	1.04785	1.05144	1.05520	1.05912	1.06322	1000121	1.07665	1.08152	1.08661	1.09191	1.09744	1.10321	0.1	1.11551		٦.	36	1.14349	1.15128
0.01	1.00001	1.00008	1.00052	1.00090	1.00137	1.00195	1.00262	1.00340	1.00429	1.00528	1.00637	1.00758		1.01031		•		1.01713					1.02336	1.03100	1.03377	1.03068	1.03974	1.04294	1.04629	1.04980	1.05348	1.05732	1.06133	1.04.000	1.05490	1.07923	1.08420	1.08938	1.09479	1.10043	1.10631	1.11244	1.11884	1.12552	1.13248	1.13975	1.14734	1.15526
Y/1 4/4	0.01	0.02	50.0	0.0	0.06	0.07	0.08	0.09	0.10	0.11	0.12	0.13	0.14	0.15	0.16	0.17	0.18	0.19	0.20	0.21	0.22	0.23	0.24	0.25	0.26	0.27	0.28	0.29	0.30	0.31	0.32	0.33	0.34	0.35	0.50	86.0	0.39	0.40	0.41	0.42	0.43	0.44	0.45	94.0	0.47	0.48	64.0	0.50

Table 1 value of correction factor, γ

0.12 0.13 0.14 0.15 0.16 0.17 0.17 0.18 0.10 0.17 0.17 0.09475 1.00954 1.00959 1.00959 1.00954 1.00959	0.19 0.20	1.01226 1	1.01202 1.0133	1.01206 1.0134	1.01245 1.0138	1.01280 1.0142	1.01326	1.01383	1.01450	1.01528	1.01617	1.01/17	1.01950	1.02084	1.02229	1.02386	1.02555	1.02737	1.02930	1.03137	1 03550	1.03835	1.04095	1.04369	1.04657	1.04961	1.05	1 05967	1.06335	1.06721	1.07124	1.07547	1 09651	1.08933	1.09438	1.09965	1.10515	1.11091	1.11691	1.12319	1.12975	1.13660	7.7.7
0.0396 0.00475 0.00562 0.0055 0.00756 0.00844 0.00377 0.00456 0.00456 0.00547 0.00556 0.00577 0.00456 0.00547 0.00572 0.00844 0.00372 0.00459 0.00557 0.00572 0.00844 0.00537 0.00457 0.00572 0.00844 0.00572 0.00847 0.00572 0.00847 0.00572 0.00847 0.00447 0.00574 0.00572 0.00847 0.00447 0.00572 0.00657 0.00472 0.00872	0.18		i.		-	-	Ė	1	-	-	÷.	<u>.</u>	-	1	-	Ť	i.	-	٠.	٠,		-	-	-	i	Ċ	٦,	-i -	-	1	i	<u> </u>		-	-	1	1	ř	-	-	i	.i .	
0.12 0.13 0.14 0.15 1.00475 1.00552 1.00655 1.00756 1.00757 1.00449 1.00534 1.00627 1.00737 1.00737 1.00749 1.00534 1.00627 1.00737 1.00749 1.00549 1.00540 1.00742 1.00749 1.00549 1		1.0097	1.00	1.00	1.00	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.02	1.02	1.02	1.02	1.02	1.02	1.02	1.03	1.04	1.04	1.04	1.04	1.0	1.05	1.06	1.06	1.07	1.00	1.08	1.09	1.09	1.10	1.10	1.11	1.11	1.12	1.13	
0.12 0.13 0.14 0.15 1.00475 1.00562 1.00655 1.007 1.00449 1.00543 1.00627 1.007 1.00449 1.00543 1.00627 1.007 1.00449 1.00547 1.00647 1.007 1.00514 1.00568 1.00662 1.007 1.00514 1.00568 1.00662 1.007 1.00514 1.00568 1.00695 1.007 1.00514 1.00696 1.007 1.00514 1.00696 1.007 1.00514 1.00696 1.007 1.00609 1.00696 1.007 1.00609 1.00696 1.007 1.00140 1.0012 1.010 1.00120 1.00696 1.007 1.00120 1.00111 1.0111 1.00120 1.00120 1.0012 1.00120 1.00120 1.00120	-	1.0086	1.0083	1.0083	1.0087	1.0090	1.0095	1	-	7	7		٦,	1	_	7	_	_	⊢ ′		٦ -	1 -	1 -	-	7		П.	→ -	-	-	-	-	٦.	1	יר	1	1	1	_	7	_	⊣.	-
0.12 0.13 0.14 1.00475 1.00562 1.00448 1.00543 1.00460 1.00547 1.00460 1.00547 1.00574 1.00547 1.00574 1.00547 1.00574 1.00568 1.00574 1.00568 1.00574 1.00569 1.00574 1.00569 1.00574 1.00569 1.00672 1.00696 1.00672 1.00696 1.00746 1.00587 1.00833 1.00112 1.00833 1.00112 1.01011 1.01567 1.01011 1.01676 1.01011 1.01676 1.01011 1.00672 1.01011 1.00		1.0075	1.0072	1.0073	1.0076	1.0079	1.008	1.008	1.009	1.010	1.011	1.012	1.015	1.015	1.017	1.018	1.020	1.022	1.023	1.025	1.028	1.032	1.035	1.037	1.040	1.043	1.046	1.050	1.057	1.060	1.064	1.068	1.0073	1.082	1.087	1.092	1.097	1.103	1.109	1.115	1.121	1.128	
0.12 0.12 377 1.00475 1.00448 1.00469 1.00469 1.00469 1.00469 1.00469 1.00469 1.00614 1.00514 1.00514 1.00514 1.00514 1.00514 1.00609 1.00609 1.001148 1.00609 1.001148 1.001488 1.001148 1.0014	0.14				7.	, ,	-			_			, ,		-		_	_				7 -			_			_	, .					_		-					1.1200	1.1266	
0.000	•	000	0	000	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.01	1.0	1.01	1.01	1.01	1.01	1.01	1.02	ď.	-i -	-	-	-	1	1	٠.	→ -	-	-	1	-	-	٠,			1	1	1	-	-	٠ .	
0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		.0047	* 0044	.0044	1.00480	1,00514	1.00557	1.00609	1.00672	1.00746	1.00830	1.00925	1.01031	1.01276	1.01415	1.01565	1.01727	1,01901	1.02087	1.02285	1.02496	1.02419	1.03206	1.03470	1.03747	1.04040	1.04347	1.04669	1.05362	1.05733	1.06121	1.06527	1.06952	1.07861	1,08345	1.08852	1.09381	1.09933	1.10510	1.11112	1.11741	1.12398	300
	• 1	37	36	200	0 0	m	-	53	6	90	25	4 0	10	5	33	8	4	2	2	0 '	7 3	1 5	2	1 10	9	35	9	9 0	7	40	53	4	90	7	10	2	8	33	7	=	4	6	

0.30	1.03096	0	0	0	9	0 0	9 (9	9 9	2 (2 0	, c) (0	0	0	0	1.05127	0	0	0	0	0	0	0	0	0	0	0	0	0	? •	; -	:-	1	1	17	17	17	-			7	7	7	1.19709
0.29	1.02890	.0288	.0289	.0292	.0295	0300	6050	.0312	0320	.0330	0340	1 03444	4000	0393	0410	0427	.0446	.0467	0488	.0511	1.05360	0561	0589	0617	0648	0680	0713	.0749	0786	.0824	0865	8060	2660	10.00	1000	1152	1208	1266	1328	1391	.1458	.1528	.1601	1.16774	.1757	1840	1.19284
0.28	1.02692	.0268	.0269	0271	0275	0279	• 0285	-0292	0299	8050.	.0519	2 0	0356	0371	0387	.0405	1.04240	.0444	0465	.0488	.0512	0537	• 05	• 05	• 06	1.06546	• 06	.07	1.07593	•01	• 08	.08	4760*	1010	1040	3:	11176	1234	96	1357	.1423	92	.1564	1639	18	1801	1.18876
0.27	1.02500			1.02522		1.02598		•	•	1.02885		1.03097	•	• •		1.03835		1.04218	1.04429		1.04891	1.05143		1.05691		•	•	•	1.07334	•	•	1.08528	•	1 00007	1 10304	•		•	, ,					1.16038	1.16816	1.17631	
0.26	1.02316	0230	0231	1.02334	0236	0240	0246	0252	0260	6970	0278	1 02020	2000	1.03299	0345	0362	0380	0400	1.04212	0443	0466	0491	1.05182	1.05460	1.05754	1.06062	1.06387	1.06728	1.07086	1.07461	1.07854	1.08266	1.08697	1 007748	1 10114	1.10630	1.11169	1-11732	1.12321	1293	1.13580	.1425	1.14956	1.15691	1.16460	.1726	1.18107
0.25	1.02139	.02	.02	•05	• 02	• 02	0.02	• 02	• 02	70.	20.	20.		1.03102	03	.03	.03	.03	• 04	.04	• 04	1.04703	•04	• 05	• 05	• 05	•06	1.06492	•06	1.07218	0.	.08	800	1 00001	4000	1.10353	000	1144	1202	1263	.1327	.1393	.1463	5	.1611	1691	.1774
0.24	1.01969	1.01953	1.01961	1.01980	1.02009	1.02050	1.02101	1.02163	1.02236	1.02320	1.02416	1.0252	1.020-1	1.02912	1.03064	1.03232	1.03410	1.03601	1.03804	1.04021	1.04251	1.04495	1.04753	1.05025	1.05312	1.05615	1.05933	1.06266	1.06617	1.06984	1.07370	1.07773	1.08195		0000	n ar	1061	1116	1174	1234	1297	1363	1432	1504	1579	1657	1.17401
0.23	1.01807	1.01789	.0179	.0181	.0184	.0188	.0193	.0199	.0206	.0214	+770.	4620.	0250	1.02731	0288	.0304	1.03223	.0341	1.03613	.0382	.0405	1.04296	.0455	1.04821	.0510	1.05405	.0571	1.06050	1.06397	.0676	1.07142	.0754	0670	1.000 L	00000	1.00823	1035	1090	1147	1206	1269	.1334	.1402	1.14735	.1547	1.16257	1.17071
0.22	1.01651								•			•			, ,	, .									•	1.05204	•			•	•			•		1.09591	•	1-10649	1.11215	1,11805	1.12422	1,13067	1,13741	1.14445	1.15181	1,15951	1.16756
0.21	1.01502	0	0	0	0	<u>،</u> د	0	0	0	0	•	•	9 0	20		0	0	0	0	0	0	0	0	0.	0	0	0	0	0	0	٠	0	0 (•	• •				17	17	-	7	7				
1/Y	0.01	0.03	40.0	0.05	0.06	0.07	0.08	60.0	0.10	0.11	0.12	0.13	110	0.16	0.17	0.18	0.19	0.20	0.21	0.22	0.23	0.24	0.25	0.26	0.27	0.28	0.29	0.30	0.31	0.32	0.33	0.34	0.35	0.30	0000	00.00	07 0	0.41	0.42	0.43	770	0.45	0.46	0.47	0.48	0.49	0.50

Table 1 $\label{eq:table_stable} \text{Value of correction factor, } \gamma$

0.40	1.05585	1.05598	1.05623	1.05661	1.05710	1.05772	1.05847	1.05934	1.06033	1.06146	1.06272	1.06410	1.06562	1.06728	1.06907	1.07101	1.07309	1.0731	1.07768	1.08020	1.08288	1.08572	1.08873	1.09190	1.09525	1.09877	1.10248	1.10638	1.11048	1.11478	1.11929	1.12402	1.12897	1-1341/	1.13961	10041-1	1.15752	70771	1 1 7 7 0 1	1 1 1 1 0 0 0	1.17809	1.18560	1.19348	1.20173	1.21038	1.21946	1.22899	1.23899	1.24950
0.39	1.05299	0531	.0533	.0537	.0541	.0547	.0555	.0563	.0573	.0584	.0596	.0610	.0625	.0641	•0659	.0678	.0698	.0720	44/0.	.0769	.0795	.0823	.0852	.0884	.0917	.0951	.0988	.1026	.1067	.1109	.1153	.1200	.1249	.1300	.1353	01410	1530	2000	1461.	1001.	.1132	.1806	.1883	.1964	.2049	.2138	.2232	.2330	.2433
0.38	1.05022	0503	.0505	.0508	.0513	.0519	.0526	.0534	.0544	.0555	.0567	.0580	.0595	.0611	.0629	.0647	.0667	.0689	.0712	.0736	.0762	.0790	.0819	.0850	.0882	.0917	.0953	0660.	.1030	.1072	.1116	.1161	.1210	.1260	.1313	14.24	1404	100	1414	01010	- 1085	.1758	.1834	.1913	1661.	.2084	.2176	.2273	.2374
0.37	1.04753	1.04761	1.04782	1.04816	1.04861	1.04919	1.04988	1.05070	1.05164	1.05270	1.05389	1.05521	1.05666	1.05823	1.05994	1.06179	1.06377	1.06589	1.06816	1.07057	1.07313	1.07585	1.07872	1.08175	1.08495	1.08832	1.09187	1.09560	1.09952	1.10363	1.10794	1.11246	1.11719	1.12215	1.12735	1 13073	1 1 1 4 4 4 4	1 1 1 1 1 1 1	1 16710	6176101	10491	1.17116	1.17864	1.18648	1.19468	1.20329	1.21230	1.22176	1.23169
0.36	1.04493		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0 '	0	0	0	0	0	0	0	0	0	0	0		٦.			7	-	7.		•			٦.	7	٦.		٦.	~	.2		
0.35	1.04240	042	.042	.042	.043	.043	.044	.045	•046	.047	.048	.049	.051	.052	.054	.056	.058	.060	.062	.064	.067	690.	1.07253	.075	.078	.081	.085	.088	.092	960.	.100	.105	.109	.114	.119	120	061.	001.	747.	0110	-155	.162	.169	.177	.185	.193	.202	.211	.220
0.34	1.03996	0300	.0401	.0404	.0408	.0414	.0450	.0428	.0437	.0447	.0458	.0470	.0484	.0499	.0516	.0533	.0552	.0572	.0594	.0617	.0642	.0668	10	.0724	.0755	.0788	.0822	.0857	.0895	.0934	.0976	.1019	.1064	1112	.1162	4171.	1226	. 200	.1585	1111	1512	.1581	.1652	.1727	.1805	.1887	.1973	.2063	-2157
0.33	1.03759			·	•	٠.	٧.		•	٦.	٠.	٠.	٠.	•	•	٠.	•	•	٠.	·	٠.	·	•	Ÿ	٠.		9	•	٠.	•	٠.	•	-		7.				7.	•	-	-	7	-	٦.	-	-		
0.32 ,	1.03531	0352	.0354	.0357	.0361	.0366	.0372	.0379	.0388	.0397	.0408	.0420	.0434	•0448	.0464	.0481	.0500	.0519	.0541	.0563	.0587	.0612	.0639	.0667	.0697	.0729	.0762	.0797	.0833	.0872	.0912	.0954	6660.	1045	.1093	5511.	1251	2621.	0161.	1)610	.1435	.1501	.1570	.1643	.1719	.1798	.1832	.1969	090
0.31	1.03310	1.03306	1.03521	1.03347	1.03385	1.03434	1.03494	1.03566	1.03650	1.03745	1.03852	1.03971	1.04102	1.04246	1.04402	1.04571	1.04752	1.04947	1.05155	1.05377	1.05613	1.05863	1.06128	1.06407	1.06703	1.07014	1.07341	1.07685	1.08046	1.08426	1.08823	1.09240	1.09677	1.10134	1.10612	1.1113	1 12:05	4077101	1 13355	00001.1	18651.1	1.14635	1.15320	1.16036	1,16785	1.17569	1.18389	1.19249	1.20149
8/b	0.01) C	0	0	0	0	0	0	_		$\overline{}$	-	-	_	_	_	_	- 4	N	N	N	N	0.24	N	N	N	OI I	N	m	m	3	m	m i	m i	m c	0 0	0.0	n .	+ 4	٠.	+	+ .	4	4	4	4	4	.4	10

VALUE OF CORRECTION FACTOR, Y

0.50	580 1.08966 588 1.08975 610 1.08998											_					_				_				_	_				_		_	_		- 1	_	_
0.49	07 1.08580 14 1.08588 35 1.08610																																				
0.48	1 1.08235 1 1.08214 1 1.08235																																				
0.47	3 1.07845 3 1.07851 7 1.07871																																				
94.0	1.07493			٦,		_	-	-	٦,	7	_	-	-	7-	' -	٦.	7 -	7 -	'-	_	_		1 -	-	7	٦.	٦.	1 -	. ~	_	_	_	_	7	-	_	_
0.45		1.0724					1.0807																	_	_												_
0.44	1.06819 1.06823 1.06840																																				
0.43	1.06497	1.06585 1.06585 1.06639	1.06706	1.06878	1.06984	1.07236	1.07382	1.07717	1.07906	1.08327	1.08560	1.08809	1.09074	1.09355	1.09968	1.10301	1.10652	1.11410	1.11819	1.12249	1.12700	1.13174	1.14192	1.14738	1.15310	1.15910	1 17104	1.17886	1.18609	1.19367	1.20161	1.20994	1.21868	1.22786	1.23749	1.24/62	1.25827
0.42	1.06184 1.06186 1.06201	1.06228 1.06268 1.06320	1.06385	1.06554	1.06775	1.06905	1.07049	1.07378	1.07563	1.07977	1.08207	1.08452	1.08712	1.08989	1.09592	1.09919	1.10265	1.11011	1.11413	1.11836	1.12280	1.12746	1.13747	1.14283	1.14846	1.15435	1 144 99	1-17376	1.18086	1.18829	1.19609	1.20426	1.21283	1.22182	1.23126	1.24117	1.25159
0.41	1.05880 1.05881 1.05895	1.05921 1.05960 1.06011	1.06074	1.06239	1.06341	1.06584	1.06725	1.07048	1.07230	1.07638	1.07864	1.08105	1.08361	1.08634	1.09227	1.09549	1 10347	1-10624	1.11020	1.11436	1.11873	1.12331	1.13315	1.13843	1.14396	1.14976	1.15282	1-16883	1.17580	1.18310	1.19075	1.19877	1.20718	1.21600	1.22525	1.23497	1.24517
1/Y	0.01	0.05	0.07	60.00	0.10	0.12	0.13	0.15	0.16	0.18	0.19	0.20	0.21	0.22	0.24	0.25	0.26	0.28	0.29	0.30	0.31	0.32	0.34	0.35	0.36	0.37	0.38	0.40	0.41	0.42	0.43	0.44	0.45	0.46	0.47	84.0	64.0

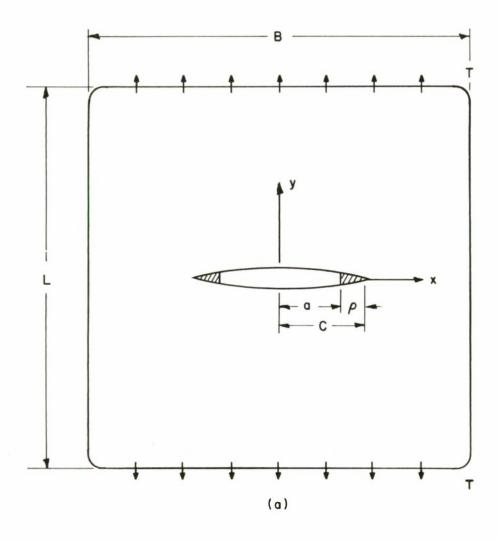
TABLE 1

VALUE OF CORRECTION FACTOR, Y

09*0 6	1.12984 1.13502																																											
0.58 0.5	1.12482 1.1		_	_	.12722	.12818	12040	13207	13371	13551	.13750	.13967	.14202 1	.14456	.14729	15335	15669	16025	.16402	.16803	.17227	.17676	.18150	.18651	.19180	.19737	.20325		.22285	.23009	.23772	.24576	. 25424	. 27262	28259	.29312	.30425	.31605	.32855	.34182	.35593	.37096	.38700	40417
0.57	1.11995	1,12041	1.12087	1.12149				12701	12860		.13231	.13443	.13673	.13921		17447	15106		.15822	.16214		.17066	.17529	.18017	.18533	.19076	.19649		.21557	.22261	.23003													
0.56	1,11522	1,11566	1.11611	1.11671	1.11747	1.11839	1.11946	1 12210	1.12346	1,12538	1.12728	1,12935	1.13160	1.13402	1.13663	1 14545	1.14541	1.14901	1,15261	1.15644	1.16048	1.16476	1.16928	1.17405	1.17908	1.18439	1.18997	1.20205	1,20856	1.21542	1.22264	1.23024	1.23823	1.25553	1.26489	1.27476	1.28517	1.29617	1.30781	1.32013	1.33318	1.34704	1.36177	1.37747
0.55	1.11064	1.1	1.1	1.1	1:1	1:1	1.			1 1	1.1	1.1	1.1	1:1	1:1	7 - 7	1.1	1	1:1	1.1	1.1	1.1	1.1	1:1	1.1	1.1	1:1	1.1	1.2	1.2	1.2	1.2	1.5	2.1	1.2	1.2	1.2	1.2	1.2	1.3	1.3	1.3	1.3	E . L
0.54	1.10619			1.1					-		1.1	1.1	7:1	1:1	7		1 - 1	1.1		1.1	1.1	1.1	1.1	1.1	1.1					1.2	1.2	1.5	I.	7 -	1.2	1.2	1.2	1.2	1.2	1. 2	1.3	1.32563	1,33930	25282
0.53	1.10187	1.10	1.10	1.10	1:10	1.10	1.10	1 - 1	1 10	1.11	1,11	1.11	1:11	1.11	1.12	71.1	1 13	1,13	1.13	1.14	1.14	1.14	1.15	1.15	1.16	1.16	1.17	1.18	1.18	1,19	1.20	1.20	1.21	1.23	1.24	1.25	1.25	1.26	1.28	1.29	1.30	_	1.3289	
0.52	1.09767						_ ,		-								-	_							_		_ ,	_	-						-							1	_	
0.51	1.09360	1.09394	1.09432	1.09484	1,09551	1.0963	1.00 (2)	1 00063	10.01	1.10258	1,10428	1,10615	1.10817	1,11035	1.11.71	111702	1.12080	1-12385	1,12710	1.13054	1.13417	1.13802	1.14207	1.14635	1.15086	1,15560	1,1605	1.17134	1.17714	1,18322	1.18961	1,19632	1.20331	1.21655	1.22672	1,23531	1.24435	1.25386	1.26387	1.27441	1,28553	1.29727	1.30967	1 22:70
4/b	0.01	0.03	0.04	0.05	90.0	0.07	0.08	0.00	1100	0.12	0,13	0.14	0.15	0.16	0.17	0.0	0.20	0.21	0.22	0.23	0.24	0.25	0.26	0.27	0.28	0.29	0,30	0.37	0.33	0.34	0.35	0.36	0.31	0.30	0,40	0.41	0.42	0.43	0.44	0.45	95.0	14.0	0.48	07.0

×12	0.61	0.62	69.0	0.64	0.65	99.0	19.0	0.68	69.0	0.70
	. 14	1.14588	-	1574	1,16355	7	٦.	1.18310	.1900	1,19734
0.02	1.14055	1.14608	1.15178	1.15769	1.16377	1.17008	1.17660	1.18336	1.19036	1.19762
0	.14	1.14645	7	1580	1.16418	٦.	٦,	1.18382	.1908	1.19812
()	14	1.14699	7	1586	1.16478	٦.		1.18448	1915	1.19883
0	.14	1.14771	٦.	1594	1.16557		. 1	1.18535	.1924	1.19976
0	• 1.4	1.14860	٦.	1603	1.16655	7		1.18642	.1935	1.20091
0	.14	1.14968	-	1615	1-16772	7	7	1.18770	.1948	1.20228
C	.14	1.15093	7	1628	1.16908	٦.		1.18920	.1964	1.20387
0	.14	1.15236	7	1643	1.17064	7	7	1.19090	.1981	1.20570
-4	.14	1.15398	7	1660	1.17240	٦.	7.	1.19283	.2001	1.20775
-	.15	1.15579	7	1679	1.17436	∵.	7	1.19498	. 2023	1.21004
_	.15	1.15779	7	1700	1.17653	7	7.	1.19735	.2048	1.21258
	1.15409	1.15998	7	1723	1.17891	7.	-: -	1.19996	-2075	1.21536
		1.16238	7	7 4 4 T	10101-1		•	1 30566	1017	1 22170
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4 -		1 1 20 7 0	•	1837	1.19065	: -	40	1.21282	2208	1.22912
4 -		1.17403	: -	1872	1-19417			1.21668	2248	1.23326
4 -) r	1.17749	: 7	1908	1,19793			1.22082	.2290	1.23770
4 0	17	1.18118	7	1947	1.20196		2	1.22525	.2336	1.24245
JA	7	1.18512		1989	1.20625			1.22997	.2385	1.24753
10		1,18930		2033	1.21081		. 2	1.23501	.2437	1.25295
IN	.18	1,19374	1	2080	1.21566		2	1.24037	.2493	1.25873
N		1.19846	10	2130	1.22082		2	1.24608	.2552	1.26488
N	.19	1.20345	2	2183	1.22628		.2	1.25214	.2615	1.27144
N	.20	1.20873	.4	2239	1.23208		5	1.25858	.2682	1.27841
N	.20	1.21431	14	2299	1.23821		.2	1.26541	.2753	1.28583
N	.21	1.22022		2362	1.24471	14	?	1.27267	.2829	1.29373
N	.21	1.22646	2	2428	1.25159		2	1.28038	.2909	1.30213
m	.22	1.23305		2498	1.25887		?	1.28856	.2995	1.31108
m	•23	1.24001	2	2573	1.26658		2	1.29725	.3086	1.32062
m	.23	1.24736	2	2651	1.27475		2	1.30649	.3182	1.33080
m	.24	1.25512	2	2735	1.28340		m i	1.31632	.3286	1.34167
3		1.26333		2823	1.29256		m i	1.32678	.3396	1.35330
m	• 26	1.27200		2916	1.30228		. u	1.33/94	.3513	1.36577
rn n	.27	1.28117	4	3112	1.31260	. r	, ,	1 26760	70000	1 20262
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١ ٠	קית	1 34892		3755	1.39037	7	7	1.44283	4642	1.48866
1	י ה ה	34285		0100	1.40674	,	. 4	1-46344	4871	1.51509
1	הע	1.37773	, ,	4076	1.42446	7	7	1.48631	5131	1.54661
h d	37	1.39366	7	4255	1.44368	7	4	1.51208	. 5436	1.58910
. 4	30	1.41078	. 4	4450	1-46465	7	, r	1.54184	5819	
- 3	14	1.42922	7	4661	1.48773		1 15	1.57802		
*		1.44918	4.	4884	1.51338		S.	1.63124		
. 4	45	1.47089	7.	5152	1.54233		9			
10	1.47408	1.49466	1.51771	5441	1.57584	9				

TABLE I $\label{eq:table_state} \text{VALUE OF CORRECTION FACTOR, } \boldsymbol{\gamma}$



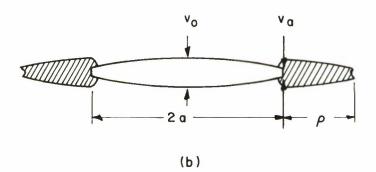


Figure 1. Model of Dugdale Crack

- (a) The Dugdale Model
- (b) The Actual Crack (Hahn's Experiment Reference 8)

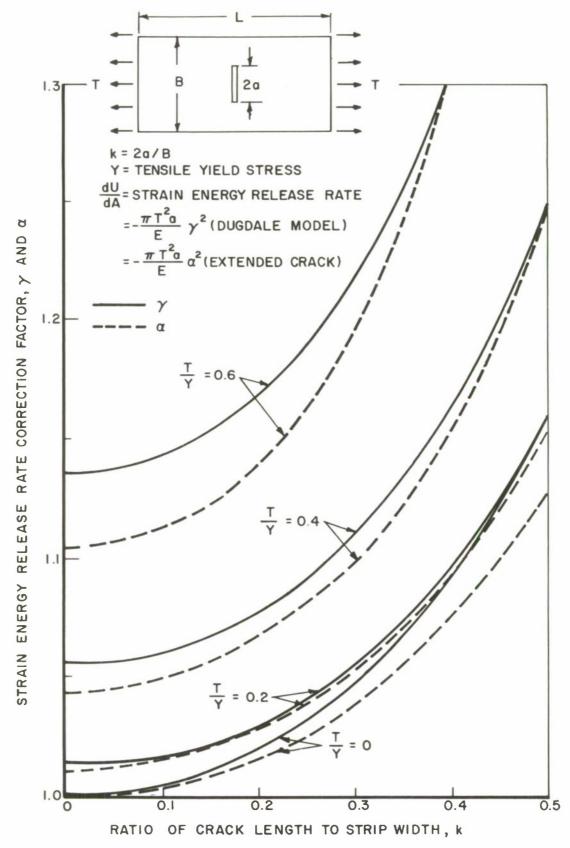


Figure 2. Variation of the Strain Energy Release Rate Correction Factors With Ratio of Crack Length to Strip Width, k

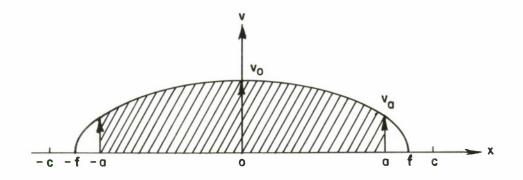


Figure 3. Displacement of The Crack Boundary

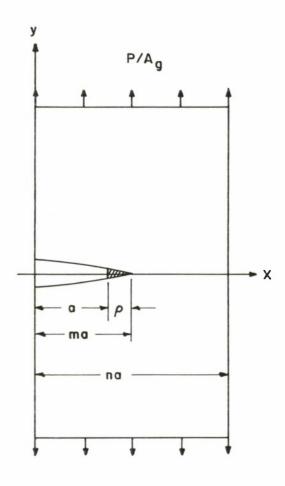


Figure 4. Dimensions of Centrally Cracked Plate Under Uniaxial Tension

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Wright-Patterson Air Force Base, Ohio		2 b. GROUP						
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IN A CENTRALLY NOTCHED PLATE SUBJECTED TO UNIAXIAL TENSION								
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By using the Dugdale model for a crack in a plate, an improved formula was derived								
for the strain energy release rate, G. The formula has the same form as the solution								
for a linear elastic plate, except a correction factor is used which corrects for both								

By using the Dugdale model for a crack in a plate, an improved formula was derived for the strain energy release rate, G. The formula has the same form as the solution for a linear elastic plate, except a correction factor is used which corrects for both the effect of yielding and the finite width of the plate. Curves are presented giving the values of the correction factor, and they indicate that the nominal stress to yield stress ratio has a pronounced effect on the strain energy release rate.

Security Classification

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